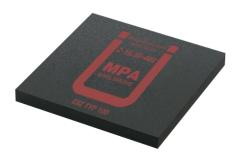


## ESZ type 100

Unreinforced elastomeric bearing with building authority **approval Z-16.32-482** 

Load capacity up to 14 N/mm<sup>2</sup>



**General information** Product description

**Deformation behaviour** Compression characteristic and bulging behaviour

Compression characteristic curve Bulging behaviour of the bearing

**Planning tools** Design tables t=10 mm

Design tables t=15 mm Design tables t=20 mm

Tender texts

**Design** Dimensions - Shape factor - Boreholes

Load capacity and rotation Rotation - twisting surcharge Transverse tensile forces

**Calculation example**Load capacity under rotation

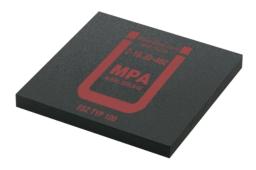
**Bearing use in practice**Installation instructions

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The user of our products is obliged to check the suitability and application possibilities for the intended purpose himself. We reserve the right to make technical changes due to new findings or product developments.

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## ESZ type 100 | for static component bearing Technical documentation



### Special advantages

- Load-bearing capacity up to 14 N/mm<sup>2</sup> (depending on format)
- Material > Vulcanisate based on EPDM rubber
- Proven in practice for decades
- DIBt approval Z-16.32-482
- Can be used as a point or strip bearing
- Maintenance-free and very durable
- Low creep behaviour
- Very good mechanical-physical characteristics

#### Intended use

The **ESZ type 100** is a compact elastomeric bearing for use as static bearing of structural elements, like steelworks, wood constructions or especially in precast concrete construction. It is used in accordance with the provisions of the general building inspectorate **approval Z-16.32-482**.

#### Deformation

Depending on the nature of the contact surfaces, an average bearing deflection of  $\leq 40\%$  can be expected at maximum permissible loads.

### **Delivery form**

#### - Precast concrete construction

Available as cuttings for all standard bearing dimensions in precast concrete construction with boreholes, cut-outs and bevelled cuts.

Nominal bearing thicknesses: 10, 15, 20, 25 and 30 mm.

### - Use for in-situ concrete

The bearing can be supplied pre-fabricated with a dummy formwork for use in in-situ concrete. The formwork can be produced for strip and point bearings. This applies to all available bearing thicknesses of 10, 15, 20, 25 and 30 mm for in-situ concrete use.

#### Temperature range

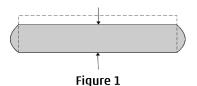
The temperature application range is between -25 °C and +50 °C.

The bearings may be exposed to temperatures of up to +70 °C for short, recurring periods of less than 8 hours.

The **ESZ type 100** elastomeric bearing can be used both indoors and outdoors.

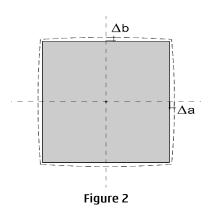
## ESZ type 100 | for static component bearing Technical documentation

The following pages show pressure compression characteristic curves and the bulging behaviour for selected bearing formats in diagram form.



The orientation diagrams make it possible to estimate the deflection as a function of the existing compressive stress. The characteristic curves were determined on reinforced concrete contact surfaces and with centric load application. The diagrams each show the evaluation at the third load on the bearing. In construction practice, the deflection may deviate from the values of the compressive compression characteristics given here as examples, depending on the characteristic of the substrate, deviations of the contact surfaces from plane parallelism and any rotations/offsets that occur.

The deflection decreases with increasing bearing footprint sizes.



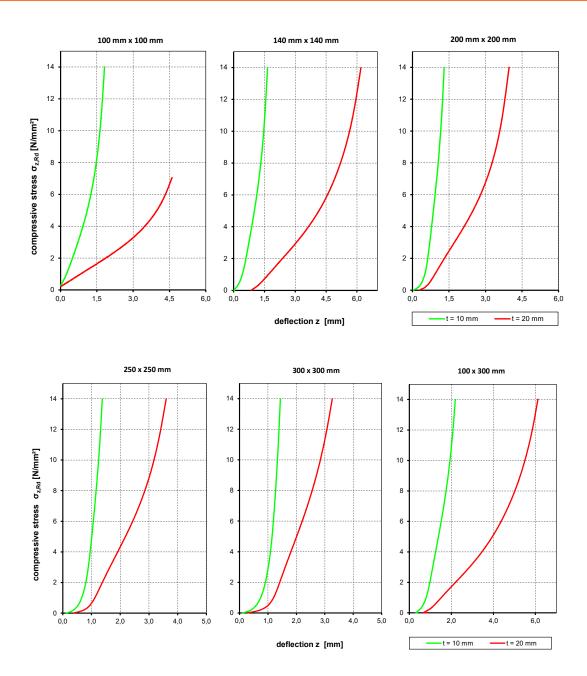
The bulging behaviour depends on the nominal bearing thickness and the permissible design compressive stress. The expansion dimensions in the diagrams shown refer to one side of the bearing, as shown in Figure 2.

The bulging behaviour is largely dependent on the **roughness** of the contact surfaces. The roughness of the concrete contact surfaces from these tests was analysed in accordance with DIN EN ISO 4287.

The arithmetic mean roughness value Ra was determined from 4 individual measurement strips > Ra = 808.5µm.

Typical roughness values are: Concrete (200-900 μm); steel (1-50 μm)

# ESZ type 100 | for static component bearing t = 10 mm and 20 mm (exemplary formats, concrete contact surface)

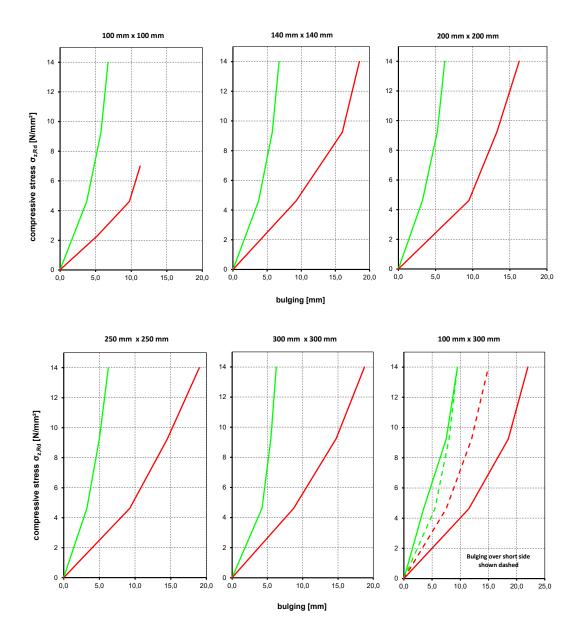


The bearing thicknesses 10 mm and 20 mm are each summarised in a diagram, where t = 10 mm is shown in green and t = 20 mm in red.

On request, we will be happy to determine the deflection and the bulging of not shown bearing formats according to our technical capabilities.

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# ESZ type 100 | for static component bearing t = 10 mm and 20 mm (exemplary formats, concrete contact surface)



The bearing thicknesses 10 mm and 20 mm are each summarised in a diagram, where t = 10 mm is shown in green and t = 20 mm in red.

On request, we will be happy to determine the deflection and the bulging of not shown bearing formats according to our technical capabilities.

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## $R_{Ld}$ [N/mm<sup>2</sup>] | Bearing thickness t = 10 mm

#### Important note:

The table shows the maximum permissible values of the load-bearing capacity with corresponding rotational capacity parallel to side b  $(\alpha_b)$  in accordance with the approval conditions and is only intended as a guide. In our opinion, the interaction between compressive stress and rotation is not taken into account in a practical manner.



As soon as holes are drilled in the bearing, the shape factor changes and therefore the entire basis for design changes.

You can conveniently carry out a specific dimensioning for your application using the ESZ dimensioning tool online.

| α <sub>b</sub> | side a |     |      |      |      |      |      |      |      | side b | [mm] |      |      |      |      |      |      |      |      |
|----------------|--------|-----|------|------|------|------|------|------|------|--------|------|------|------|------|------|------|------|------|------|
| [‰]            | [mm]   | 70  | 80   | 90   | 10 0 | 110  | 120  | 130  | 140  | 150    | 160  | 170  | 180  | 190  | 200  | 230  | 250  | 270  | 300  |
| 40,0           | 50     | -   | -    | -    | 8,3  | 8,6  | 8,8  | 9,1  | 9,3  | 9,4    | 9,6  | 9,7  | 9,9  | 10,0 | 10,1 | 10,4 | 10,6 | 10,7 | 10,9 |
| 40,0           | 60     | 1   | -    | -    | 9,4  | 9,8  | 10,1 | 10,4 | 10,6 | 10,9   | 11,1 | 11,3 | 11,5 | 11,6 | 11,8 | 12,4 | 12,8 | 13,2 | 13,7 |
| 40,0           | 70     | 8,8 | 9,4  | 9,9  | 10,4 | 10,9 | 11,2 | 11,6 | 11,9 | 12,5   | 13,0 | 13,5 | 13,8 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0           | 80     |     | 10,1 | 10,7 | 11,3 | 11,8 | 12,6 | 13,4 | 14,0 | 14,0   | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0           | 90     |     |      | 11,5 | 12,3 | 13,4 | 14,0 | 14,0 | 14,0 | 14,0   | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0           | 100    |     |      |      | 13,7 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0   | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0           | 110    |     |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0   | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 37,5           | 120    |     |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0   | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 34,6           | 130    |     |      |      |      |      |      | 14,0 | 14,0 | 14,0   | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 32,1           | 140    |     |      |      |      |      |      |      | 14,0 | 14,0   | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 30,0           | 150    |     |      |      |      |      |      |      |      | 14,0   | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 28,1           | 160    |     |      |      |      |      |      |      |      |        | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 26,5           | 170    |     |      |      |      |      |      |      |      |        |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 25,0           | 180    |     |      |      |      |      |      |      |      | }      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 23,7           | 190    |     | {    |      |      |      |      |      |      |        |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 22,5           | 200    |     |      |      |      |      |      |      |      | }      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 21,4           | 210    |     |      |      |      |      |      |      |      |        |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 |
| 20,5           | 220    |     |      |      |      |      |      |      |      |        |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 |
| 19,6           | 230    |     |      |      |      |      |      |      |      | }      |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 |
| 18,8           | 240    |     |      |      |      |      |      |      |      | }      |      |      |      |      |      |      | 14,0 | 14,0 | 14,0 |
| 18,0           | 250    |     |      |      |      |      |      |      |      |        |      |      |      |      |      |      | 14,0 | 14,0 | 14,0 |
| 17,3           | 260    |     |      |      |      |      |      |      |      |        |      |      |      |      |      |      |      | 14,0 | 14,0 |
| 16,7           | 270    |     |      |      |      |      |      |      |      |        |      |      |      |      |      |      |      | 14,0 | 14,0 |
| 16,1           | 280    |     |      |      |      |      |      |      |      |        |      |      |      |      |      |      |      |      | 14,0 |
| 15,5           | 290    |     |      |      |      |      |      |      |      |        |      |      |      |      |      |      |      |      | 14,0 |
| 15,0           | 300    |     |      |      |      |      |      |      |      |        |      |      |      |      |      |      |      |      | 14,0 |

Bearing thickness t = 10 mm: Limit dimension of shorter bearing side  $a_{max} = 300$  mm

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## F<sub>d,max</sub>[kN] | bearing thickness t = 10 mm

#### Important note:

The table shows the maximum permissible values of the load-bearing capacity with corresponding rotational capacity parallel to side b  $(\alpha_b)$  in accordance with the approval conditions and is only intended as a guide. In our opinion, the interaction between compressive stress and rotation is not taken into account in a practical manner.



As soon as holes are drilled in the bearing, the shape factor changes and therefore the entire basis for design changes.

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| αь   | side a |    |    |  |     |     |     |     |     | side b | [mm] |     |     |     |     |     |     |      |      |
|------|--------|----|----|--|-----|-----|-----|-----|-----|--------|------|-----|-----|-----|-----|-----|-----|------|------|
| [‰]  | [mm]   | 70 | 80 | 90   | 100 | 110 | 120 | 130 | 140 | 150    | 160  | 170 | 180 | 190 | 200 | 230 | 250 | 270  | 300  |
| 40,0 | 50     | -  | -  | -  | 42  | 47  | 53  | 59  | 65  | 71     | 77   | 83  | 89  | 95  | 101 | 120 | 132 | 144  | 163  |
| 40,0 | 60     | -  | -  | -  | 57  | 65  | 73  | 81  | 89  | 98     | 106  | 115 | 124 | 132 | 141 | 171 | 193 | 214  | 246  |
| 40,0 | 70     | 43 | 53 | 63   | 73  | 84  | 94  | 105 | 117 | 131    | 146  | 160 | 174 | 186 | 196 | 225 | 245 | 265  | 294  |
| 40,0 | 80     |    | 65 | 77   | 90  | 104 | 121 | 140 | 157 | 168    | 179  | 190 | 202 | 213 | 224 | 258 | 280 | 302  | 336  |
| 40,0 | 90     |    |    | 93   | 111 | 133 | 151 | 164 | 176 | 189    | 202  | 214 | 227 | 239 | 252 | 290 | 315 | 340  | 378  |
| 40,0 | 100    |    |    |  | 137 | 154 | 168 | 182 | 196 | 210    | 224  | 238 | 252 | 266 | 280 | 322 | 350 | 378  | 420  |
| 40,0 | 110    |    |    |  |     | 169 | 185 | 200 | 216 | 231    | 246  | 262 | 277 | 293 | 308 | 354 | 385 | 416  | 462  |
| 37,5 | 120    |    |    |  |     |     | 202 | 218 | 235 | 252    | 269  | 286 | 302 | 319 | 336 | 386 | 420 | 454  | 504  |
| 34,6 | 130    |    |    |  | 1   |     |     | 237 | 255 | 273    | 291  | 309 | 328 | 346 | 364 | 419 | 455 | 491  | 546  |
| 32,1 | 140    |    |    |  |     |     |     |     | 274 | 294    | 314  | 333 | 353 | 372 | 392 | 451 | 490 | 529  | 588  |
| 30,0 | 150    |    |    |  |     |     |     |     |     | 315    | 336  | 357 | 378 | 399 | 420 | 483 | 525 | 567  | 630  |
| 28,1 | 160    |    |    |  |     |     |     |     |     |        | 358  | 381 | 403 | 426 | 448 | 515 | 560 | 605  | 672  |
| 26,5 | 170    |    |    |  | 1   |     |     |     |     |        |      | 405 | 428 | 452 | 476 | 547 | 595 | 643  | 714  |
| 25,0 | 180    |    |    |  |     |     |     |     |     |        |      |     | 454 | 479 | 504 | 580 | 630 | 680  | 756  |
| 23,7 | 190    |    |    |  | 1   |     |     |     |     |        |      |     |     | 505 | 532 | 612 | 665 | 718  | 798  |
| 22,5 | 200    |    |    |  |     |     |     |     |     |        |      |     |     |     | 560 | 644 | 700 | 756  | 840  |
| 21,4 | 210    |    |    |  |     |     |     |     |     |        |      |     |     |     |     | 676 | 735 | 794  | 882  |
| 20,5 | 220    |    |    | <u>:                                    </u> |     |     |     |     |     |        |      |     |     |     |     | 708 | 770 | 832  | 924  |
| 19,6 | 230    |    |    |  |     |     |     |     |     |        |      |     |     |     |     | 741 | 805 | 869  | 966  |
| 18,8 | 240    |    |    |  |     |     |     |     |     |        |      |     |     |     |     |     | 840 | 907  | 1008 |
| 18,0 | 250    |    |    |  |     |     |     |     |     |        |      |     |     |     |     |     | 875 | 945  | 1050 |
| 17,3 | 260    |    |    | <u> </u>                                     |     |     |     |     |     |        |      |     |     |     |     |     |     | 983  | 1092 |
| 16,7 | 270    |    |    | <u> </u>                                     |     |     |     |     |     |        |      |     |     |     |     |     |     | 1021 | 1134 |
| 16,1 | 280    |    |    | <u> </u>                                     |     |     |     |     |     |        |      |     |     |     |     |     |     |      | 1176 |
| 15,5 | 290    |    |    |  | 1   |     |     |     |     |        |      |     |     |     |     |     |     |      | 1218 |
| 15,0 | 300    |    |    | :  | {   |     |     |     | {   |        |      |     |     |     |     |     |     |      | 1260 |

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## $R_{\perp d}$ [N/mm<sup>2</sup>] | Bearing thickness t = 15 mm

#### Important note:

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| αь   | side a |     |     |     | •   |      |      |      |      |      | sic  | le b [mn | n]   |      |      |      |      |      |      |      |      |      |
|------|--------|-----|-----|-----|-----|------|------|------|------|------|------|----------|------|------|------|------|------|------|------|------|------|------|
| [‰]  | [mm]   | 80  | 90  | 100 | 110 | 120  | 13 0 | 140  | 150  | 160  | 170  | 18 0     | 190  | 200  | 230  | 250  | 270  | 300  | 350  | 400  | 450  | 500  |
| 40,0 | 80     | 6,5 | 6,9 | 7,3 | 7,7 | 8,0  | 8,2  | 8,5  | 8,7  | 8,9  | 9,1  | 9,3      | 9,4  | 9,6  | 10,0 | 10,2 | 10,4 | 10,7 | 11,0 | 11,3 | 11,5 | 11,7 |
| 40,0 | 90     | -   | 7,4 | 7,8 | 8,2 | 8,6  | 8,9  | 9,2  | 9,4  | 9,7  | 9,9  | 10,1     | 10,3 | 10,5 | 10,9 | 11,2 | 11,5 | 11,8 | 12,5 | 13,2 | 13,7 | 14,0 |
| 40,0 | 100    |     |     | 8,3 | 8,7 | 9,1  | 9,5  | 9,8  | 10,1 | 10,4 | 10,6 | 10,9     | 11,1 | 11,3 | 11,8 | 12,4 | 13,0 | 13,7 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 110    | }   |     |     | 9,2 | 9,6  | 10,0 | 10,4 | 10,7 | 11,0 | 11,3 | 11,6     | 11,8 | 12,3 | 13,5 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 120    |     |     |     |     | 10,1 | 10,5 | 10,9 | 11,3 | 11,6 | 12,0 | 12,6     | 13,2 | 13,7 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 130    | -   |     |     | :   |      | 11,0 | 11,4 | 11,8 | 12,5 | 13,2 | 13,8     | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 140    |     |     |     |     |      |      | 11,9 | 12,8 | 13,6 | 14,0 | 14,0     | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 150    |     |     |     |     |      |      |      | 13,7 | 14,0 | 14,0 | 14,0     | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 160    |     |     |     |     |      |      |      |      | 14,0 | 14,0 | 14,0     | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 39,7 | 170    | -   |     |     |     |      |      |      |      |      | 14,0 | 14,0     | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 37,5 | 180    |     |     |     |     |      |      |      |      |      |      | 14,0     | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 35,5 | 190    | }   |     |     |     | [    |      |      |      |      |      |          | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 33,8 | 200    |     |     |     |     |      |      |      |      |      |      |          |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 32,1 | 210    | -   |     |     | :   |      |      |      |      |      |      |          |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 30,7 | 220    | -   |     |     |     |      |      |      |      |      |      |          |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 29,3 | 230    |     |     |     |     |      |      |      |      |      |      |          |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 28,1 | 240    | -   |     |     |     |      |      |      |      |      |      |          |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 27,0 | 250    |     |     |     |     |      |      |      |      |      |      |          |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 26,0 | 260    | - { |     |     |     |      |      |      |      |      |      |          |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 25,0 | 270    | -   |     |     |     |      |      |      |      |      |      |          |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 24,1 | 280    | }   |     |     |     |      |      |      |      |      |      |          |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 23,3 | 290    |     |     |     |     |      |      |      |      |      |      |          |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 22,5 | 300    | -   |     |     |     |      |      |      |      |      |      |          |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 19,3 | 350    |     |     |     |     |      |      |      |      |      |      |          |      |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 |
| 16,9 | 400    |     |     |     |     |      |      |      |      |      |      |          |      |      |      |      |      |      |      | 14,0 | 14,0 | 14,0 |
| 15,0 | 450    |     |     |     |     |      |      |      |      |      |      |          |      |      |      |      |      |      |      |      | 14,0 | 14,0 |

Bearing thickness t = 15 mm: Limit dimension of shorter bearing side  $a_{max} = 450$  mm

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## F<sub>d,max</sub>[kN] | Bearing thickness t = 15 mm

#### Important note:

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| αь   | side a |    |    |      |     |     |      |     |     |     | sic | le b [mr | n]  |     |     |     |      |      |      |      |      |      |
|------|--------|----|----|------|-----|-----|------|-----|-----|-----|-----|----------|-----|-----|-----|-----|------|------|------|------|------|------|
| [‰]  | [mm]   | 80 | 90 | 10 0 | 110 | 120 | 13 0 | 140 | 150 | 160 | 170 | 18 0     | 190 | 200 | 230 | 250 | 270  | 300  | 350  | 400  | 450  | 500  |
| 40,0 | 80     | 42 | 50 | 59   | 67  | 76  | 86   | 95  | 104 | 114 | 124 | 134      | 144 | 154 | 184 | 204 | 225  | 256  | 309  | 362  | 415  | 469  |
| 40,0 | 90     |    | 60 | 71   | 81  | 93  | 104  | 116 | 127 | 139 | 151 | 164      | 176 | 189 | 227 | 252 | 278  | 318  | 393  | 474  | 554  | 630  |
| 40,0 | 100    |    |    | 83   | 96  | 110 | 123  | 137 | 152 | 166 | 181 | 196      | 211 | 226 | 272 | 311 | 350  | 410  | 490  | 560  | 630  | 700  |
| 40,0 | 110    |    |    |      | 111 | 127 | 143  | 160 | 177 | 194 | 212 | 230      | 247 | 270 | 341 | 385 | 416  | 462  | 539  | 616  | 693  | 770  |
| 40,0 | 120    |    |    |      |     | 146 | 164  | 184 | 203 | 224 | 246 | 273      | 301 | 328 | 386 | 420 | 454  | 504  | 588  | 672  | 756  | 840  |
| 40,0 | 130    |    |    |      |     | 1   | 186  | 208 | 231 | 261 | 292 | 323      | 346 | 364 | 419 | 455 | 491  | 546  | 637  | 728  | 819  | 910  |
| 40,0 | 140    |    |    |      |     |     |      | 234 | 268 | 304 | 333 | 353      | 372 | 392 | 451 | 490 | 529  | 588  | 686  | 784  | 882  | 980  |
| 40,0 | 150    |    |    |      |     |     |      |     | 308 | 336 | 357 | 378      | 399 | 420 | 483 | 525 | 567  | 630  | 735  | 840  | 945  | 1050 |
| 40,0 | 160    |    |    |      |     |     |      |     |     | 358 | 381 | 403      | 426 | 448 | 515 | 560 | 605  | 672  | 784  | 896  | 1008 | 1120 |
| 39,7 | 170    |    | {  |      |     | }   |      |     |     |     | 405 | 428      | 452 | 476 | 547 | 595 | 643  | 714  | 833  | 952  | 1071 | 1190 |
| 37,5 | 180    |    |    |      |     | }   |      |     |     |     |     | 454      | 479 | 504 | 580 | 630 | 680  | 756  | 882  | 1008 | 1134 | 1260 |
| 35,5 | 190    |    |    |      |     |     |      |     |     |     |     |          | 505 | 532 | 612 | 665 | 718  | 798  | 931  | 1064 | 1197 | 1330 |
| 33,8 | 200    |    |    |      |     |     |      |     |     |     |     |          |     | 560 | 644 | 700 | 756  | 840  | 980  | 1120 | 1260 | 1400 |
| 32,1 | 210    |    |    |      |     |     |      |     |     |     |     |          |     |     | 676 | 735 | 794  | 882  | 1029 | 1176 | 1323 | 1470 |
| 30,7 | 220    |    | }  |      |     |     |      |     |     |     |     |          |     |     | 708 | 770 | 832  | 924  | 1078 | 1232 | 1386 | 1540 |
| 29,3 | 230    |    | }  |      |     |     |      |     |     |     |     |          |     |     | 741 | 805 | 869  | 966  | 1127 | 1288 | 1449 | 1610 |
| 28,1 | 240    |    |    |      |     |     |      |     |     |     |     |          |     |     |     | 840 | 907  | 1008 | 1176 | 1344 | 1512 | 1680 |
| 27,0 | 250    |    |    |      |     |     |      |     |     |     |     |          |     |     |     | 875 | 945  | 1050 | 1225 | 1400 | 1575 | 1750 |
| 26,0 | 260    |    |    |      |     |     |      |     |     |     |     |          |     |     |     |     | 983  | 1092 | 1274 | 1456 | 1638 | 1820 |
| 25,0 | 270    |    |    |      |     |     |      |     |     |     |     |          |     |     |     |     | 1021 | 1134 | 1323 | 1512 | 1701 | 1890 |
| 24,1 | 280    |    |    |      |     | 1   |      |     |     |     |     |          |     |     |     |     |      | 1176 | 1372 | 1568 | 1764 | 1960 |
| 23,3 | 290    |    |    |      |     |     |      |     |     |     |     |          |     |     |     |     |      | 1218 | 1421 | 1624 | 1827 | 2030 |
| 22,5 | 300    |    |    |      |     | {   |      |     |     |     |     |          |     |     |     |     |      | 1260 | 1470 | 1680 | 1890 | 2100 |
| 19,3 | 350    |    | }  |      |     |     |      |     |     |     |     |          |     |     |     |     |      |      | 1715 | 1960 | 2205 | 2450 |
| 16,9 | 400    |    | }  |      |     | [   |      |     |     |     |     |          |     |     |     |     |      | :    |      | 2240 | 2520 | 2800 |
| 15,0 | 450    |    |    |      |     |     |      |     |     |     |     |          |     |     |     |     |      |      |      |      | 2835 | 3150 |

## $R_{\perp d}$ [N/mm<sup>2</sup>] | Bearing thickness t = 20 mm

#### Important note:

The table shows the maximum permissible values of the load-bearing capacity with corresponding rotational capacity parallel to side b  $(\alpha_b)$  in accordance with the approval conditions and is only intended as a guide. In our opinion, the interaction between compressive stress and rotation is not taken into account in a practical manner.

As soon as holes are drilled in the bearing, the shape factor changes and therefore the entire basis for design changes.

You can conveniently carry out a specific dimensioning for your application using the ESZ dimensioning tool online.



| αb   | side a |     |     |     |     |     |      |     |     |      | sic  | ie b [mr | n]   |      |      |      |      |      |      |      |      |      |      |      |
|------|--------|-----|-----|-----|-----|-----|------|-----|-----|------|------|----------|------|------|------|------|------|------|------|------|------|------|------|------|
| [‰]  | [mm]   | 80  | 90  | 100 | 110 | 120 | 13 0 | 140 | 150 | 160  | 170  | 180      | 190  | 200  | 230  | 250  | 270  | 300  | 350  | 400  | 450  | 500  | 550  | 600  |
| 40,0 | 80     | 4,7 | 5,0 | 5,3 | 5,6 | 5,8 | 6,0  | 6,2 | 6,4 | 6,5  | 6,7  | 6,8      | 6,9  | 7,0  | 7,3  | 7,5  | 7,6  | 7,8  | 8,1  | 8,3  | 8,5  | 8,6  | 8,7  | 8,8  |
| 40,0 | 90     |     | 5,4 | 5,7 | 6,0 | 6,3 | 6,5  | 6,7 | 6,9 | 7,1  | 7,3  | 7,4      | 7,6  | 7,7  | 8,0  | 8,2  | 8,4  | 8,7  | 9,0  | 9,2  | 9,4  | 9,6  | 9,8  | 9,9  |
| 40,0 | 100    |     |     | 6,1 | 6,4 | 6,7 | 6,9  | 7,2 | 7,4 | 7,6  | 7,8  | 8,0      | 8,2  | 8,3  | 8,7  | 9,0  | 9,2  | 9,4  | 9,8  | 10,1 | 10,4 | 10,6 | 10,7 | 10,9 |
| 40,0 | 110    |     |     |     | 6,7 | 7,1 | 7,4  | 7,6 | 7,9 | 8,1  | 8,3  | 8,5      | 8,7  | 8,9  | 9,4  | 9,6  | 9,9  | 10,2 | 10,6 | 11,0 | 11,2 | 11,5 | 11,7 | 11,9 |
| 40,0 | 120    |     |     |     |     | 7,4 | 7,7  | 8,0 | 8,3 | 8,6  | 8,8  | 9,0      | 9,2  | 9,4  | 10,0 | 10,3 | 10,5 | 10,9 | 11,4 | 11,8 | 12,3 | 12,8 | 13,3 | 13,7 |
| 40,0 | 130    |     |     |     |     |     | 8,1  | 8,4 | 8,7 | 9,0  | 9,3  | 9,5      | 9,7  | 9,9  | 10,5 | 10,9 | 11,1 | 11,5 | 12,3 | 13,2 | 13,9 | 14,0 | 14,0 | 14,0 |
| 40,0 | 140    |     |     |     |     |     |      | 8,8 | 9,1 | 9,4  | 9,7  | 9,9      | 10,2 | 10,4 | 11,1 | 11,4 | 11,7 | 12,5 | 13,7 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 150    |     |     |     |     |     |      |     | 9,4 | 9,8  | 10,1 | 10,4     | 10,6 | 10,9 | 11,6 | 12,0 | 12,7 | 13,7 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 160    |     |     |     |     |     |      |     |     | 10,1 | 10,4 | 10,7     | 11,0 | 11,3 | 12,2 | 13,0 | 13,8 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 170    |     |     |     | 1   |     |      |     |     |      | 10,8 | 11,1     | 11,4 | 11,7 | 13,1 | 13,9 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 180    |     |     |     |     |     |      |     |     |      | }    | 11,5     | 11,8 | 12,3 | 13,9 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 190    |     |     |     |     |     |      |     |     |      | {    |          | 12,4 | 13,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 200    |     |     |     |     |     |      |     |     |      | {    |          |      | 13,7 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 210    |     |     |     |     |     |      |     |     |      | }    |          |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 40,0 | 220    |     |     |     |     |     |      |     |     |      | {    |          |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 39,1 | 230    |     |     |     |     |     |      |     |     |      | }    |          |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 37,5 | 240    |     |     |     |     |     |      |     |     |      | {    |          |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 36,0 | 250    |     |     |     |     |     |      |     |     |      | }    |          |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 34,6 | 260    |     |     |     |     |     |      |     |     |      | }    |          |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 33,3 | 270    |     |     |     |     |     |      |     |     |      | {    |          |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 32,1 | 280    |     |     |     |     |     |      |     |     |      | {    |          |      |      | -    |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 31,0 | 290    |     |     |     |     |     |      |     |     |      | }    |          |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 30,0 | 300    |     |     |     |     |     |      |     |     |      | }    |          |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 25,7 | 350    |     |     |     |     |     |      |     |     |      | 1    |          |      |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 22,5 | 400    |     |     |     |     |     |      |     |     |      | {    |          |      |      |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 | 14,0 |
| 20,0 | 450    |     |     |     |     |     |      |     |     |      | 3    |          |      |      |      |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 |
| 18,0 | 500    |     |     |     |     |     |      |     |     |      | }    |          |      |      | }    |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 |
| 16,4 | 550    |     |     |     |     |     |      |     |     |      | }    |          |      |      |      |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 |
| 15,0 | 600    |     |     |     |     |     |      |     |     |      | {    |          |      |      | {    |      |      |      |      |      | 14,0 | 14,0 | 14,0 | 14,0 |

Bearing thickness t = 20 mm: Limit dimension of shorter bearing side  $a_{max} = 600$  mm

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## F<sub>d,max</sub>[kN] | bearing thickness t = 20 mm

#### Important note:

The table shows the maximum permissible values of the load-bearing capacity with corresponding rotational capacity parallel to side b  $(\alpha_b)$  in accordance with the approval conditions and is only intended as a guide. In our opinion, the interaction between compressive stress and rotation is not taken into account in a practical manner.

As soon as holes are drilled in the bearing, the shape factor changes and therefore the entire basis for design changes.

You can conveniently carry out a specific dimensioning for your application using the ESZ dimensioning tool online.



| αь   | side a |    |          |          |          |          |          |             |          |     | sic      | ie b [mr | n]       |     |          |          |          |          |      |      |      |               |               |      |
|------|--------|----|----------|----------|----------|----------|----------|-------------|----------|-----|----------|----------|----------|-----|----------|----------|----------|----------|------|------|------|---------------|---------------|------|
| [‰]  | [mm]   | 80 | 90       | 100      | 110      | 120      | 130      | 140         | 150      | 160 | 170      | 180      | 190      | 200 | 230      | 250      | 270      | 300      | 350  | 400  | 450  | 500           | 550           | 600  |
| 40,0 | 80     | 30 | 36       | 43       | 49       | 56       | 62       | 69          | 76       | 83  | 91       | 98       | 105      | 113 | 135      | 150      | 165      | 188      | 227  | 266  | 305  | 345           | 385           | 424  |
| 40,0 | 90     |    | 44       | 51       | 59       | 68       | 76       | 85          | 93       | 102 | 111      | 120      | 129      | 139 | 167      | 186      | 205      | 234      | 283  | 332  | 382  | 432           | 483           | 533  |
| 40,0 | 100    |    |          | 61       | 70       | 80       | 90       | 101         | 111      | 122 | 133      | 144      | 155      | 166 | 201      | 224      | 247      | 283      | 343  | 404  | 466  | 528           | 590           | 653  |
| 40,0 | 110    |    |          |          | 82       | 93       | 105      | 118         | 130      | 143 | 156      | 169      | 182      | 196 | 237      | 265      | 293      | 336      | 408  | 482  | 556  | 631           | 706           | 782  |
| 40,0 | 120    |    |          |          |          | 107      | 121      | 135         | 150      | 165 | 180      | 195      | 211      | 226 | 275      | 308      | 341      | 392      | 477  | 565  | 664  | 770           | 878           | 984  |
| 40,0 | 130    |    |          |          |          | <u> </u> | 137      | 153         | 170      | 187 | 205      | 222      | 240      | 259 | 315      | 353      | 391      | 450      | 560  | 686  | 810  | 910           | 1001          | 1092 |
| 40,0 | 140    |    |          |          |          | <u> </u> |          | 172         | 191      | 210 | 230      | 250      | 271      | 292 | 356      | 400      | 444      | 525      | 670  | 784  | 882  | 980           | 1078          | 1176 |
| 40,0 | 150    |    |          |          |          |          |          |             | 212      | 234 | 257      | 280      | 303      | 326 | 399      | 451      | 516      | 615      | 735  | 840  | 945  | 1050          | 1155          | 1260 |
| 40,0 | 160    |    |          |          |          |          |          |             |          | 259 | 284      | 309      | 335      | 362 | 449      | 522      | 595      | 672      | 784  | 896  | 1008 | 1120          | 1232          | 1344 |
| 40,0 | 170    |    |          |          |          | <u> </u> |          | <u> </u>    |          |     | 312      | 340      | 369      | 398 | 512      | 592      | 643      | 714      | 833  | 952  | 1071 | 1190          |               | 1428 |
| 40,0 | 180    |    |          |          |          |          |          |             |          |     |          | 371      | 403      | 443 | 574      | 630      | 680      | 756      | 882  | 1008 | 1134 | 1260          | 1386          | 1512 |
| 40,0 | 190    |    |          |          |          |          |          | <u> </u>    |          |     |          |          | 446      | 495 | 612      | 665      | 718      | 798      | 931  | 1064 | 1197 | 1330          | 1463          | 1596 |
| 40,0 | 200    |    |          |          |          |          |          |             |          |     |          |          |          | 547 | 644      | 700      | 756      | 840      | 980  | 1120 | 1260 | 1400          | 1540          | 1680 |
| 40,0 | 210    |    |          |          |          | <u> </u> |          |             |          |     |          |          |          |     | 676      | 735      | 794      | 882      | 1029 | 1176 | 1323 | 1470          | $\overline{}$ | 1764 |
| 40,0 | 220    |    |          |          |          | <u> </u> |          | <u> </u>    |          |     |          |          |          |     | 708      | 770      | 832      | 924      | 1078 | 1232 | 1386 | 1540          | _             | 1848 |
| 39,1 | 230    |    |          |          |          | <u> </u> |          |             |          |     |          |          | <b></b>  |     | 741      | 805      | 869      | 966      | 1127 | 1288 | 1449 | 1610          | 1771          | 1932 |
| 37,5 | 240    |    |          | <u> </u> |          | <u> </u> | <u> </u> |             |          |     |          |          |          |     |          | 840      | 907      | 1008     | 1176 |      | 1512 |               | 1848          | 2016 |
| 36,0 | 250    |    |          | <u> </u> |          | <u> </u> |          | <u> </u>    |          |     |          |          |          |     |          | 875      | 945      | 1050     | 1225 | 1400 | 1575 | $\overline{}$ | 1925          | 2100 |
| 34,6 | 260    |    |          |          | <u> </u> | <b></b>  |          | <u> </u>    | <u> </u> |     |          |          |          |     |          |          | 983      |          | 1274 | •    | 1638 |               |               | 2184 |
| 33,3 | 270    |    |          |          |          | }        |          | <u> </u>    |          |     |          |          |          |     |          | <u> </u> | 1021     | 1134     | 1323 | 1512 | 1701 | 1890          | 2079          | 2268 |
| 32,1 | 280    |    | <u> </u> |          | <u> </u> | <b>}</b> | <u> </u> | <u> </u>    | <u> </u> |     | <u> </u> |          |          |     | <u> </u> | <u> </u> |          | 1176     | 1372 | 1568 | 1764 | 1960          | 2156          | 2352 |
| 31,0 | 290    |    |          |          |          | <u> </u> | <u> </u> | <u> </u>    |          |     |          |          |          |     | <u> </u> | <u> </u> | <u> </u> | 1218     | 1421 | 1624 | 1827 | 2030          | 2233          | 2436 |
| 30,0 | 300    |    |          |          |          | <b>}</b> |          | <u> </u>    |          |     |          |          |          |     | <b>!</b> |          |          | 1260     | 1470 | 1680 | 1890 | 2100          | $\overline{}$ | 2520 |
| 25,7 | 350    |    |          |          |          | <b>}</b> | 1        | <u> </u>    |          |     |          |          |          |     | !        |          |          |          | 1715 | 1960 | 2205 | 2450          |               | 2940 |
| 22,5 | 400    |    |          | <u> </u> | <u> </u> | <b>↓</b> |          |             | <u> </u> |     |          |          |          |     |          | <b>└</b> |          |          |      | 2240 | 2520 | 2800          | 3080          | 3360 |
| 20,0 | 450    |    |          | <u> </u> | -        | <u> </u> |          |             |          |     |          |          | <b>[</b> |     |          |          |          |          |      | -    | 2835 | 3150          | -             | 3780 |
| 18,0 | 500    |    |          |          |          | <u> </u> |          | <del></del> |          |     |          |          |          |     | -        |          |          | <u> </u> |      |      | 3150 | 3500          |               | 4200 |
| 16,4 | 550    |    |          |          |          |          | 1        |             |          |     | }        |          |          |     | }        |          |          |          |      |      | 3465 | 3850          | 4235          | 4620 |
| 15,0 | 600    |    |          |          |          | }        |          |             |          |     |          |          | }        |     | }        | 1        |          |          |      |      | 3780 | 4200          | 4620          | 5040 |

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## ESZ type 100 | for static component bearing Technical documentation

#### - for use between precast reinforced concrete elements

Supply and installation of unreinforced elastomeric bearings with national technical approval between precast reinforced concrete elements. Can be used up to a compressive stress of 14 N/mm<sup>2</sup> depending on the format. The proof of the usability of the bearings must be provided.

| Bearing type:                       | ESZ type 100 with national technical approval Z-16.32-482 |   |
|-------------------------------------|---|---|
| Bearing thickness (10/15/20/25/30): | mm  |   |
| Format a x b:                       | mm xmm  |   |
| Boreholes:                          | Number Diameter   |   |
| Quantity:                           | Piece   | 7 |
| Proof of purchase:                  | ESZ Wilfried Becker GmbH                                  |   |
|                                     | Weilerhöfe 1, 41564 Kaarst                                |   |
|                                     | Phone: +49 2131 75 81 00: info@esz-hecker.de              |   |

#### - for use as an in-situ concrete point bearing

Supply and installation of non-reinforced elastomeric bearings with national technical approval as in-situ concrete point bearings. Can be used up to a compressive stress of 14 N/mm² depending on the format. The proof of the usability of the bearings must be provided.

| Bearing type:                       | ESZ type 100 with national technical approval Z-16.32-482 |
|-------------------------------------|---|
| Bearing thickness (10/15/20/25/30): | mm  |
| Format a x b:                       | mm xmm  |
| Format incl. blank formwork         |   |
| aGxaG:                              | mmxmm   |
| Boreholes:                          | Number Diameter   |
| Quantity:                           | Piece   |
| Proof of purchase:                  | ESZ Wilfried Becker GmbH                                  |
|                                     | Weilerhöfe 1, 41564 Kaarst                                |
|                                     | Phone: +49 2131 75 81 00: info@esz-hecker de              |

### - for use as an in-situ concrete strip bearing

Supply and installation of unreinforced elastomeric bearings with national technical approval as in-situ concrete strip bearings. Can be used up to a compressive stress of 14 N/mm² depending on the format.

The mathematical proof of the usability of the bearings must be provided.

| Bearing type:                       | ESZ type 100 with national technical approval Z-16.32-482 |
|-------------------------------------|---|
| Bearing thickness (10/15/20/25/30): | mm  |
| Format a:                           | mm  |
| Format incl. blind formwork aG:     | mm  |
| Boreholes:                          | Number Diameter   |
| Quantity:                           | metres  |
| Proof of purchase:                  | ESZ Wilfried Becker GmbH                                  |
|                                     | Weilerhöfe 1, 41564 Kaarst                                |
|                                     | Phone: +49 2131 75 81 00: info@esz-hecker.de              |

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### Conditions > Dimensions of the bearings and permissible bores (according to abZ section 2.1.1)

Thickness of the bearing t = 10 to 30 mm

 $t \le a/5$  with  $t_{max} = 30$  mm  $t \ge a/30$  with  $t_{min} = 10$  mm

The following applies to rectangular, point bearings:

 $a \ge 70 \text{ mm}, b \ge 70 \text{ mm}$ 

The following applies to strip-shaped bearings:

 $a \ge 50 \text{ mm}, b \ge 100 \text{ mm}$ 

The following applies to round bearings:

r ≥ 40 mm

with

t thickness of the unloaded bearing

a shorter side of the bearing

**b** longer side of the bearing

In Table 1 of the general technical approval, the load-bearing capacities are assigned to different shape factor ranges as a design function.

The **shape factor S** for rectangular bearings is

calculated as follows:

 $S = \frac{a \cdot b}{2 \cdot t \cdot (a + b)}$ 

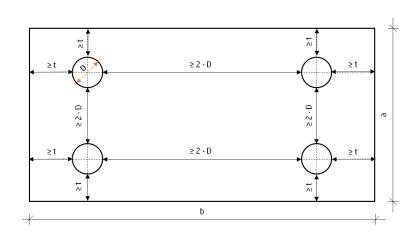
The **shape factor S** for round bearings is calculated as follows:

 $s_{mod} = \frac{r}{\sqrt{8} \cdot t}$ 

**Boreholes** (base area and lateral surfaces) must still be taken into account (=deducted) in the calculation!

Up to four boreholes are permitted per bearing, whereby the area of the boreholes may not exceed 10 % of the total area of the bearing.

The distance between the holes must be at least 2  $\times$  D. A minimum edge distance of the bearing thickness t must be maintained for the holes. The maximum diameter of the hole is D = 50 mm.



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### Calculation of the load-bearing capacity and consideration of the angle of rotation incl. surcharges

The load-bearing capacity of the **ESZ type 100** is limited by the manufacturer to **14 N/mm²**, although significantly higher load-bearing capacities are specified according to the design function in Table 1 of the approval, depending on the shape factor.

The vertical load on an elastomeric bearing leads to a **centric** load concentration and, in conjunction with a support torsion, to an **eccentric** load concentration.

The simultaneous occurrence of compressive stress and rotation must be taken into account when dimensioning an elastomeric bearing and its suitability for use must be verified accordingly.

The resulting effects on the neighbouring components must also be considered.

For the bearing design, the shear stresses from the vertical compression and the torsion are superimposed.

Elastomeric bearings allow shear deformation, but they must not be used to absorb constant external shear forces.

The angle of rotation of the adjacent components must be determined by adding the following influences:

- obliqueness with 10 ‰
- unevenness with 625/a ‰

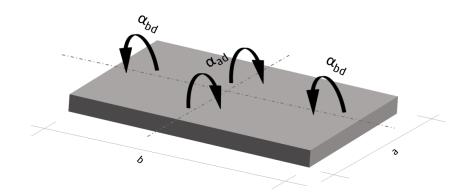
In the case of torsion across both sides of the bearing, the allowances for angular torsion are added proportionally to the respective design data (torsion due to component deformations) from the structural analysis.

In addition to the regulations listed in the approval, the following formula are used for the design of the ESZ type 100 to calculate the load-bearing capacity, taking into account support torsion (if the entire support situation is known, modifications are possible after consultation with ESZ):

$$\text{Rectangular bearing} \qquad \sigma_{z,Rd} = \left[f_{tRd} - \alpha_{bd} \cdot \frac{G}{2} \cdot (\frac{a}{t})^2 - \alpha_{ad} \cdot \frac{G}{2} \cdot (\frac{b}{t})^2\right] \cdot \frac{c}{t} \cdot \eta_2$$

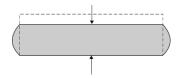
$$\text{Round bearing} \qquad \qquad \sigma_{z,Rd} = \left[ f_{tRd} - \alpha_d \cdot \frac{3 \cdot G}{8} \, N / mm^2 \cdot (\frac{d}{t})^2 \right] \cdot \frac{d}{t} \cdot \eta_2$$

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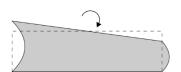
$$\alpha_{b,max} = \frac{450 \cdot t}{a} \le 48 \%_0 \qquad \alpha_{a,max} = \frac{450 \cdot t}{b} \le 48 \%_0$$

$$\alpha_{Resultierende} = \sqrt{\alpha_{a,max^2} + \alpha_{b,max^2}} \ \leq 48 \ \%_0$$



The interaction of load action and angular torsion leads to a reduction in the load-bearing capacity, which must be verified by a mathematical analysis.

Deviations from the plane parallelism and unevenness of the contact surfaces of the adjacent components are treated mathematically as planned rotations. Geometric imperfections and deviations from the plane parallelism of the contact surfaces must be recognised with at least 0.01 rad [= 10 %] and added to the calculated value of the bearing torsion.



If no more precise verification is provided, unevenness of the contact surfaces must be taken into account with 625/a [‰] and considered mathematically in the same way as planned rotations. Bearing side a is always the shorter bearing side.

If a cast-in-place concrete component is concreted onto the bearing or the contact surface is steel, this value can be halved.

### Information on transverse tensile forces in the bearing joint

The **ESZ type 100** elastomeric bearing is practically incompressible. As a result, the bearing expands transversely under compressive load at constant volume. The bearing is more or less restricted in its transverse expansion by the neighbouring components - depending on the surface properties of the component.

Roughness and surface friction are decisive influencing factors here. If the adjacent surfaces now counteract the lateral expansion of the elastomer bearing, this inevitably results in shear stresses in the joint, which lead to tensile stresses in the adjacent material and compressive stresses in the rubber.

These so-called adhesive tensile stresses in the adjacent (concrete) component are unfavourable because they can lead to damage such as edge spalling.

They increase with increasing elastomer thickness and should not be confused with splitting tensile stresses, which only become effective at a certain depth and occur with any type of partial surface load.

The reinforcement for the transverse tensile forces in reinforced concrete components should therefore be arranged as close as possible to the bearing.

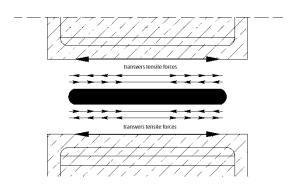


Fig.1: Representation of the transverse tensile

#### Calculation of transverse tensile forces according to ESZ - Information on the calculation assumptions

When determining the transverse tensile forces, the formula equation differs from the "old" calculation according to DIN 4141-Part 15 to the formula equation according to DIBt approval. The function for the calculation according to DIBt approval is linear, the function according to the old DIN is progressive. From a bearing side length a of approx. 320 mm, the functional equations intersect, i.e. the distinction only becomes relevant from a bearing thickness > 15 mm. From this bearing side length, the course of the curve according to DIN assumptions becomes significantly less favourable than the course of the straight line according to DIBt approval. This means that the transverse tensile forces are correspondingly higher.

For this reason, ESZ considers both functional equations when calculating the transverse tensile forces and gives the less favourable one as a recommendation for the design of the reinforcement.

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Calculation of transverse tensile forces according to "old" DIN 4141 part 15

$$\mathbf{Z_a} = \mathbf{1}, \mathbf{5} \cdot \mathbf{F}_{\mathbf{z}, \mathsf{max}, \mathsf{d}} \cdot \mathbf{t} \cdot \mathbf{b} \cdot \mathbf{10}^{-5}$$

$$\mathbf{Z_b} = \mathbf{1}, \mathbf{5} \cdot \mathbf{F}_{\mathsf{z,max,d}} \cdot \mathbf{t} \cdot \mathbf{a} \cdot \mathbf{10}^{-5}$$

Calculation of transverse tensile forces according to DIBt approval

$$\mathbf{Z_a} = \mathbf{1}, \mathbf{5} \cdot \mathbf{E}_{\perp \mathbf{d}} \cdot \mathbf{a} \cdot \mathbf{t} \cdot \mathbf{10^{-3}}$$

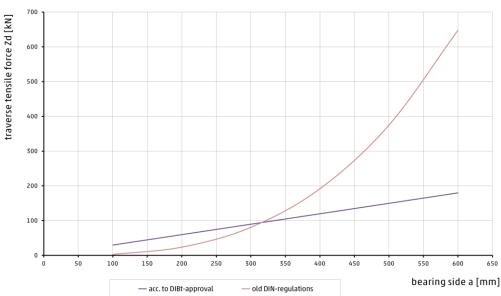
$$Z_b = 1, 5 \cdot E_{\perp d} \cdot b \cdot t \cdot 10^{\text{-}3}$$

Calculation of transverse tensile forces according to ESZ

$$Z_a = max(1.5 \cdot E_{\perp d} \cdot a \cdot t \cdot 10^{-3}; 1.5/mm^2 \cdot F_{z,max,d} \cdot t \cdot b \cdot 10^{-5})$$

$$Z_b = max(1, 5 \cdot E_{\perp d} \cdot b \cdot t \cdot 10^{\text{-3}} \text{; 1,5/mm}^2 \cdot \text{F}_{\text{z,max,d}} \cdot t \cdot \text{a} \cdot 10^{\text{-5}} \text{)}$$

evaluation of transverse tensile force | Approval against old regulations (DIN 4141-Teil 15)



### Calculation example of the transverse tensile forces in the bearing joint with centric loading

| <b>Z</b> <sub>b</sub> = | [KN]   | a = 100  mm<br>b = 200  mm<br>$t = 15 \text{ mm}$ $F_{z,max,d} = 300 \text{ kN}$<br>$E_{\perp d} = 15 \text{ N/mm}^2$      |
|-------------------------|--|--|
| E <sub>10</sub> =       | the bearing plane [N/mm²]  | $Z_{a} = max(1.5 \cdot 15 \cdot 100 \cdot 15 \cdot 10^{-3}; 1.5/\text{mm}^{2} \cdot 300 \cdot 15 \cdot 200 \cdot 10^{-5})$ |
| Fz <sub>,max,d</sub> =  | Design value of the maximum support force in the z-direction [N/mm²] | $\mathbf{Z_a} = \mathbf{33.8 \ kN}$  |
| a =                     | shorter bearing side [mm]  | $Z_{b} = max(1, 5 \cdot 15 \cdot 200 \cdot 15 \cdot 10^{-3}; 1,5/mm^{2} \cdot 300 \cdot 15 \cdot 100 \cdot 10^{-5})$       |
| t =                     | Bearing thickness [mm]   | $\mathbf{Z_b} = \mathbf{67,5 \ kN}$  |

## Character meanings in the formula

| f <sub>tRd</sub> | = | Calculated value of the internal resistance of the bearing [N/mm²]  |
|------------------|---|---|
| G                | = | Shear modulus [N/mm²]   |
| a                | = | shorter side of the bearing [mm]                                    |
| b                | = | Longer side of the bearing [mm]                                     |
| t                | = | Bearing thickness [mm]  |
| $\alpha_{bd}$    | = | Angular rotation around the axis parallel to the bearing side b [‰] |
| $\alpha_{ad}$    | = | Angular rotation around the axis parallel to the bearing side a [‰] |
| С                | = | Mainly loaded bearing side of a rectangular bearing [mm]            |
| <b>η</b> 2       | = | Aspect ratio coefficient  |

| b/a | $\eta_2$ |
|-----|----------|
| 1   | 0,208    |
| 1,5 | 0,231    |
| 2   | 0,246    |
| 3   | 0,267    |
| 4   | 0,282    |
| 6   | 0,299    |
| 8   | 0,307    |
| 10  | 0,313    |
| ∞   | 0,333    |

Table 1:

 $\eta_2$  as a function of the aspect ratio b/a as a table of values

(Intermediate values may be interpolated linearly)

$$f_{tRd} = \frac{R_{\perp d}}{\eta_2} \cdot \frac{t}{a}$$

 $f_{tRd}$  is the calculated value of the internal resistance of the bearing and is used to calculate the permissible compressive stress  $\sigma_{z,Rd}$ 

|                          | Shape factor range S<br>(S, S borehole Or Smod) | Function for determining the design value of the load-bearing capacity $R_{\perp d}$ [N/mm $^2$ ] |
|--------------------------|---|---|
| Point and strip bearings | 0.83 - 2.33                                     | $R_{\perp d} = 5.3805 \cdot S - 0.6536$   |
|                          | 2.33 – 2.50                                     | $R_{\perp d} = 10.635 \cdot S - 12.89$  |
|                          | 2.50 – 5.00                                     | $R_{\perp d} = 8.4004 \cdot S - 7.3293$   |
|                          | ≥ 5.00  | $R_{\perp d} = 34.7$  |

#### Table 2

 $R_{\perp d}$  = Rated value of the associated load-bearing capacity of the bearing [N/mm<sup>2</sup>] perpendicular to the bearing plane as a function of the shape factor S at a compression  $\varepsilon = 40 \%$ .

### **Initial assumptions**

| F <sub>z,max,d</sub>    | = | 185     | kN    |
|-------------------------|---|---------|-------|
| $f_{tRd}$               | = | Formula | N/mm² |
| a                       | = | 130     | mm    |
| b                       | = | 150     | mm    |
| t                       | = | 15      | mm    |
| G                       | = | 1.2     | N/mm² |
| α <sub>Statics</sub>    | = | 5.2     | ‰     |
| αobliqueness            | = | 10      | ‰     |
| α <sub>Unevenness</sub> | = | 4.8     | ‰     |
| $\alpha_{bd \ total}$   | = | 20      | ‰     |
| $\eta_2$                | = | 0.215   |       |

In this calculation example, a rotation around the axis parallel to the bearing side b  $(\alpha_{bd})$  is calculated. The bearing has no holes.

### Calculation wax

$$S = \frac{130 \cdot 150}{2 \cdot 15 \cdot (130 + 150)} = 2.32$$

$$R_{\perp d} = 5.3805 \cdot S - 0.6536 = 17.17 \cdot 2.32 - 0.6536 = 11.83 \text{ N/mm}^2$$

$$f_{tRd} = \frac{11.83}{0.215} \cdot \frac{15}{130} = 6.35 \text{ N/mm}^2$$

$$\sigma_{z,Rd} = \left[6.35 - 0.02 \cdot \frac{1.2}{2} \cdot \left(\frac{130}{15}\right)^2 - 0 \cdot \frac{1.2}{2} \cdot \left(\frac{150}{15}\right)^2\right] \cdot \frac{130}{15} \cdot 0.215 = \mathbf{10}.\,\mathbf{15}\,\mathbf{N/mm^2}$$

$$\sigma_{z,m} = \frac{185.000}{130 \cdot 150} = 9.49 \text{ N/mm}^2$$

 $\sigma_{z,Rd} = 10.14 \text{ N/mm}^2 \ge \sigma_m = 9.49 \text{ N/mm}^2 > \text{Proof provided!}$ 

## ESZ type 100 | with national technical approval Technical documentation

- The environmental influences must be checked with regard to possible damage to the bearings.
- Elastomer bearings and bearing surfaces must be free of dirt. Loose particles are not permitted.
- The bearing surfaces must be free of ice and snow, grease, solvents, oils or release agents. This must be ensured by suitable measures.
- The bearing surfaces must be carefully deburred to protect the bearing.
- The alignment of the bearing surfaces must be checked. If necessary, the support surfaces must be reworked to bring them into the planned condition.
- The alignment of the bearing surfaces must be checked. If necessary, the support surfaces must be reworked to bring them into the planned condition.
- Individual surface imperfections must not exceed 100 mm<sup>2</sup> and must not deviate from the surrounding surface by more than 2.5 mm in depth. The total area of the surface imperfections must not exceed 10 %.
- The bearing areas must be designed in accordance with the design-specific technical specifications and standards. Generally, edge distances must be provided. The elastomeric bearing should always be located within the reinforcement, even after expansion due to compressive stress.
- When using the bearings on steel contact surfaces, the steel surfaces should be at least 25 mm larger all round than the bearing.
- If the elastomeric bearings are tamped underneath, particular attention must be paid to the quality of the mortar. Elastomeric bearings must not be overloaded at certain points. The load of the structure to be supported by the bearings must not be applied directly to the bearing solely via wedges, unless a sufficiently rigid steel plate is interposed to distribute the load. The wedges must be removed again once the padding material has hardened.
- The lateral surfaces of the bearings must not be hindered in their planned deformation.
- Each component must be separated horizontally and vertically from the neighbouring components by joints in such a way that the intended support (statics) can be effective. It should be noted that joint fillings, e.g. joint compounds, foam profiles or mineral wool or foam panels, can impair the deformability. In the case of in-situ concrete, proper production of the bed joint must be ensured.
- In the case of horizontally movable components, it must be checked whether fixed points or fixed zones must be arranged to determine the zero point of movement of the component to be stored. It should be noted that unintended fixed points can have a detrimental effect on component storage.
- The arrangement of several storage units on top of each other (stacking) is not permitted.

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